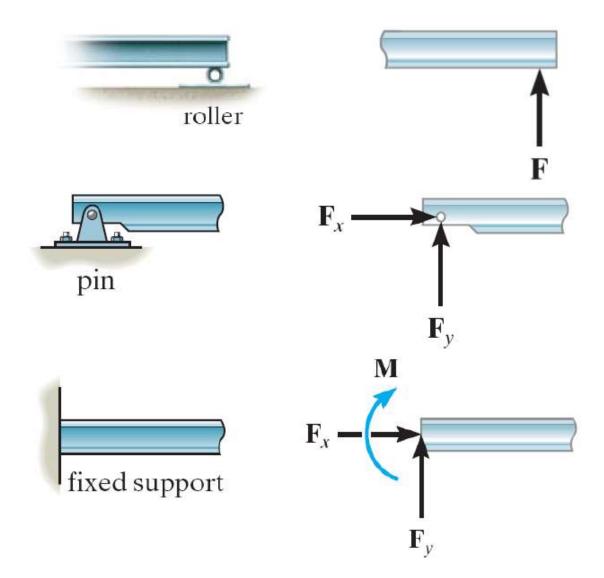
# Module 2(contd...)

- Module 2
- Types of supports -
  - Problems involving point loads and uniformly distributed loads only.
- Force systems in space
  - Degrees of freedom –
  - Free body diagram –
  - Equations of equilibrium -
  - -Simple resultant and Equilibrium problems.

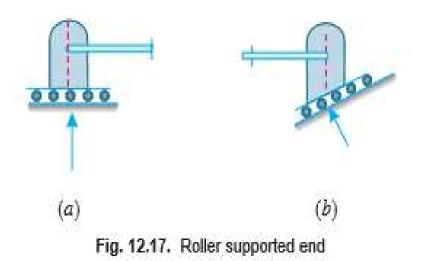
- Equilibrium
- Equilibrium of a body is a condition in which the resultants of all forces acting on the body is zero.

• Types of Supports



#### 12.14. ROLLER SUPPORTED BEAMS

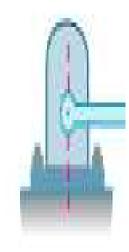
In such a case, the end of a beam is supported on rollers, and the reaction on such an end is always *normal to the support*, as shown in Fig. 12.17 (a) and (b). All the steel trusses, of the bridges, have one of their ends as supported on rollers.



The main advantage, of such a support, is that the beam can move easily towards left or right, on account of expansion or contraction due to change in temperature.

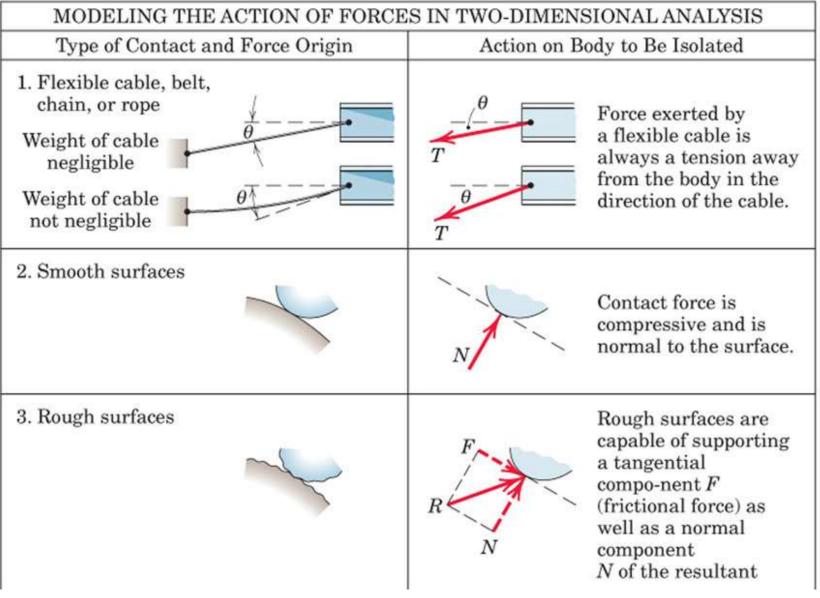
# 12.15. HINGED BEAMS

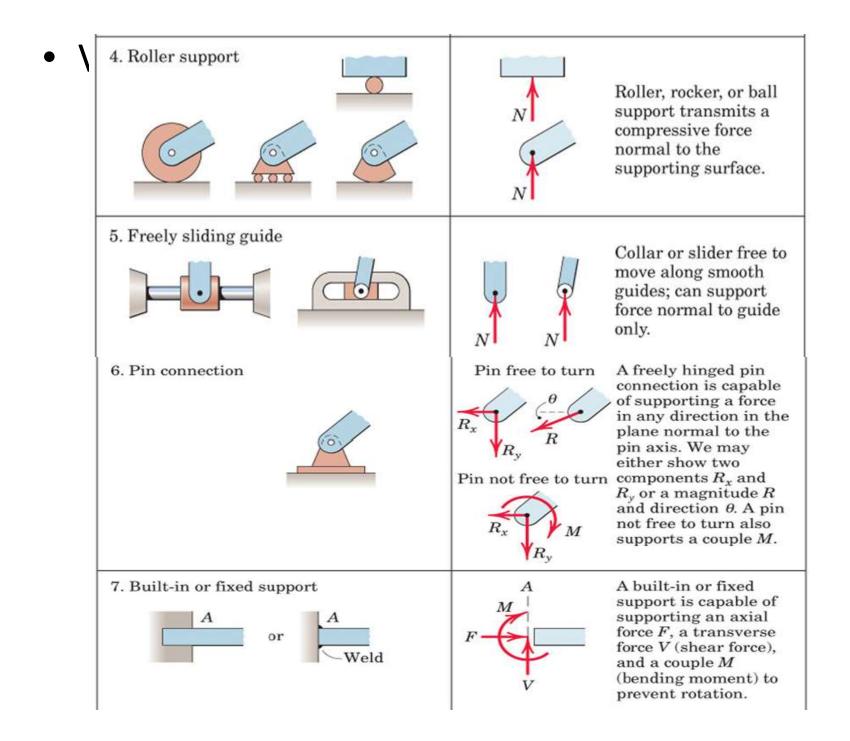
In such a case, the end of a beam is hinged to the support as shown in Fig. 12.18. The reaction on such an end may be *horizontal*, *vertical* or *inclined*, depending upon the type of loading. All the steel trusses of the bridges have one of their end roller supported, and the other hinged.

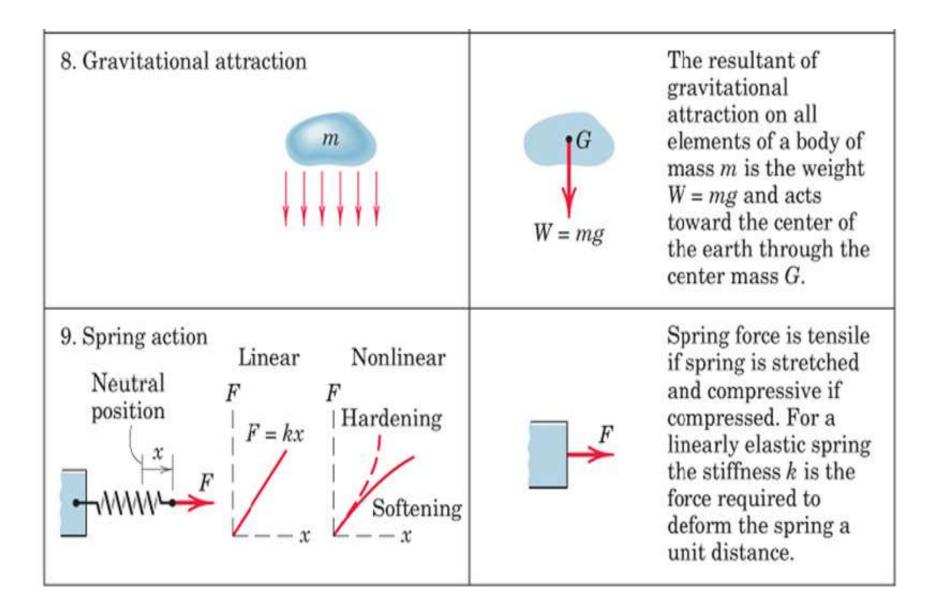


## Fig. 12.18. Hinder

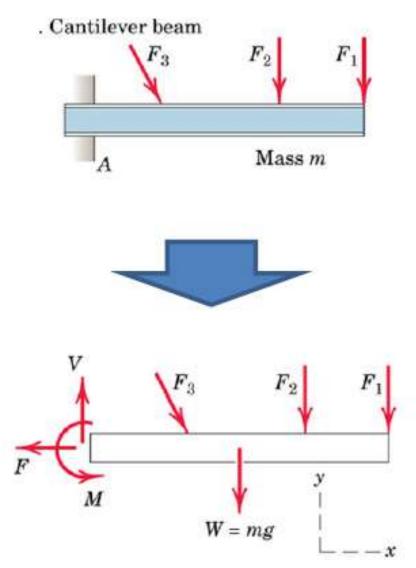
## • Various Supports







• Free body diagram



CATEGORIES OF EQUILIBRIUM IN TWO DIMENSIONS		
Force System	Free-Body Diagram	Independent Equations
1. Collinear	$\mathbf{F}_1$ $\mathbf{F}_2$ $\mathbf{F}_3$ $x$	$\Sigma F_x = 0$
2. Concurrent at a point	$\mathbf{F}_1$ $\mathbf{F}_2$ $\mathbf{F}_2$ $\mathbf{F}_3$ $\mathbf{F}_3$	$\Sigma F_x = 0$ $\Sigma F_y = 0$
3. Parallel	$\mathbf{F}_{2}$ $\mathbf{F}_{1}$ $\mathbf{F}_{2}$ $\mathbf{F}_{3}$ $\mathbf{F}_{4}$ $\mathbf{F}_{4}$	$\Sigma F_x = 0$ $\Sigma M_z = 0$
4. General	$F_1$ $F_2$ $F_3$ $y$ $F_4$ $F_4$ $F_4$	$\Sigma F_x = 0 \qquad \Sigma M_z = 0$ $\Sigma F_y = 0$

- Types of Beams
- Cantilever
- Simply Supported Beam
- Continuous Beam
- Types of Loads
- Concentrated loads W
- Uniformly distributed Loads w.x at centroid
- Uniformly varying load area enclosed by loading patern

- Reactions at supports can be determined by considering the system of forces including applied forces and reactions in equilibrium.
- Use eqns of equilibirium

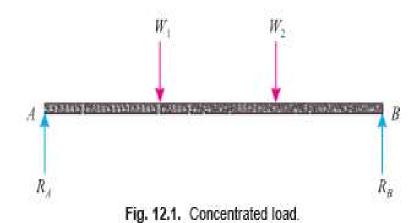
### 12.2. TYPES OF LOADING

Though there are many types of loading, yet the following are important from the subject point of view :

- 1. Concentrated or point load,
- 2. Uniformly distributed load,
- 3. Uniformly varying load.

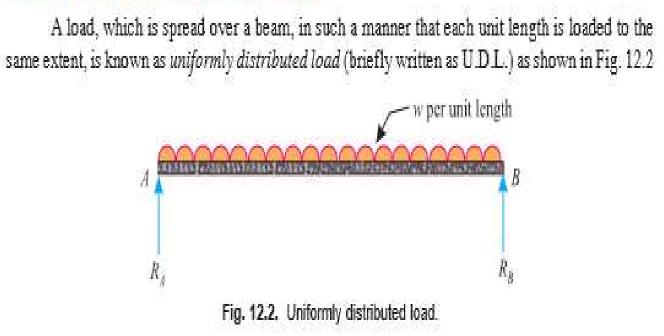
#### 12.3. CONCENTRATED OR POINT LOAD

A load, acting at a point on a beam is known as a *concentrated or a point load* as shown in Fig. 12.1.



In actual practice, it is not possible to apply a load at a point (*i.e.*, at a mathematical point), as it must have some contact area. But this area being so small, in comparison with the length of the beam, is negligible.

## 12.4. UNIFORMLY DISTRIBUTED LOAD



The total uniformly distributed load is assumed to act at the centre of gravity of the load for all sorts of calculations.

THE OWNER WE WARD DESIGNATION.

## 12.5. UNIFORMLY VARYING LOAD

A load, which is spread over a beam, in such a manner that its extent varies uniformly on each unit length (say from  $w_1$  per unit length at one support to  $w_2$  per unit length at the other support) is known as uniformly varying load as shown in Fig. 12.3.

Sometimes, the load varies from zero at one support to w at the other. Such a load is also called triangular load.

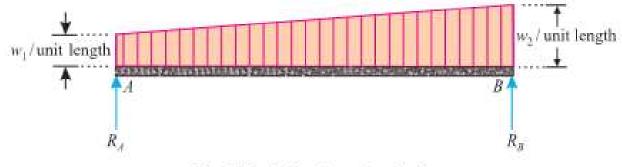
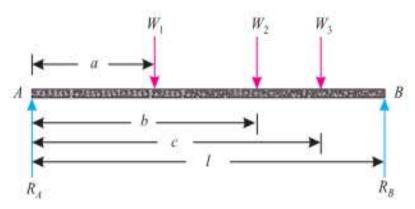
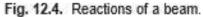


Fig. 12.3. Uniformly varying load.

Note : A beam may carry any one of the above-mentioned load system, or a combinations of the two or more.





Consider a \*simply supported beam AB of span l, subjected to point loads  $W_1$ ,  $W_2$  and  $W_3$  at distances of a, b and c, respectively from the support A, as shown in Fig. 12.4

Let  $R_A = \text{Reaction at } A$ , and

 $R_{B} = \text{Reaction at } B.$ 

We know that sum of the clockwise moments due to loads about A

$$= W_1 a + W_2 b + W_3 c$$
 ...(*i*)

and anticlockwise moment due to reaction  $R_{\scriptscriptstyle B}$  about A

$$= R_B l$$
 ....(ii)

Now equating clockwise moments and anticlockwise moments about A,

$$R_{B} l = W_{1} a + W_{2} b + W_{3} c \qquad \dots (\because \Sigma M = 0)$$

or

$$R_{B} = \frac{W_{1}a + W_{2}b + W_{3}c}{l} \qquad ...(iii)$$

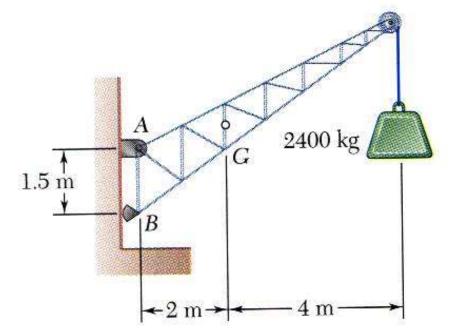
Since the beam is in equilibrium, therefore

$$\begin{split} R_A + R_B &= W_1 + W_2 + W_3 & \dots (\because \Sigma V = 0) \\ R_A &= (W_1 + W_2 + W_3) - R_B \end{split}$$

and

17

- A fixed crane has a mass of 1000 kg and is used to lift a 2400 kg crate. It is held in place by a pin at *A* and a rocker at *B*. The center of gravity of the crane is located at *G*.
- Determine the components of the reactions at *A* and *B*.



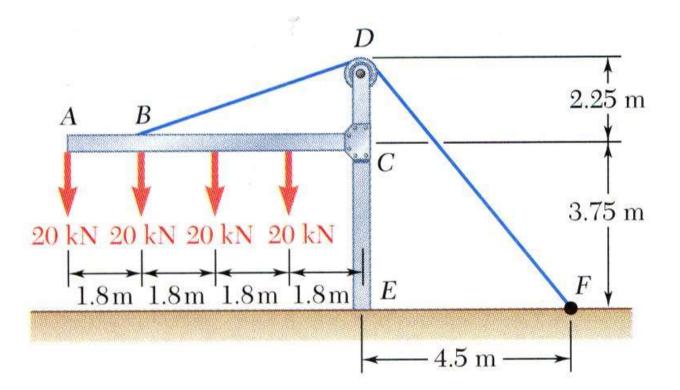
• Determine *B* by solving the equation for the sum of the moments of all forces about *A*.  $\sum M_A = 0: + B(1.5m) - 9.81 \text{ kN}(2m)$   $R = \pm 10$ 

 $-23.5 \,\mathrm{kN}(6\mathrm{m}) = 0$ 

$$B = +107.1 \,\mathrm{kN}$$

- Determine the reactions at A by solving the equations for the sum of all horizontal forces and all vertical forces.
- $\sum F_x = 0$ :  $A_x + B = 0$   $A_x = -107.1 \text{kN}$   $\sum F_y = 0$ :  $A_y - 9.81 \text{kN} - 23.5 \text{kN} = 0$   $A_y = +33.3 \text{kN}$   $A_x = -107.1 \text{kN}$   $A_y = -107.1 \text{kN}$  $A_y = -1$

• The frame supports part of the roof of a small building. The tension in the cable is 150 kN. Determine the reaction at the fixed end *E*.



• Solve 3 equilibrium equations for the reaction force components and couple.

$$\sum F_x = 0: \quad E_x + \frac{4.5}{7.5} (150 \,\text{kN}) = 0 \qquad \qquad E_x = -90.0 \,\text{kN}$$
$$\sum F_y = 0: \quad E_y - 4(20 \,\text{kN}) - \frac{6}{7.5} (150 \,\text{kN}) = 0$$
$$E_y = +200 \,\text{kN}$$

$$\sum M_E = 0: +20 \text{kN}(7.2 \text{ m}) + 20 \text{kN}(5.4 \text{ m}) + 20 \text{kN}(1.8 \text{ m}) + 20 \text{kN}(3.6 \text{ m}) + 20 \text{kN}(1.8 \text{ m}) - \frac{6}{7.5} (150 \text{kN}) 4.5 \text{ m} + M_E = 0 \xrightarrow{A \text{ B}} (150 \text{ kN}) 4.5 \text{ m} + M_E = 0 \xrightarrow{A \text{ B}} (150 \text{ kN}) 4.5 \text{ m} + M_E = 0 \xrightarrow{A \text{ B}} (150 \text{ kN}) 4.5 \text{ m} + M_E = 0 \xrightarrow{A \text{ B}} (150 \text{ kN}) 4.5 \text{ m} + M_E = 0 \xrightarrow{A \text{ B}} (150 \text{ kN}) 4.5 \text{ m} + M_E = 0 \xrightarrow{A \text{ B}} (150 \text{ kN}) 4.5 \text{ m} + M_E = 0 \xrightarrow{A \text{ B}} (150 \text{ kN}) 4.5 \text{ m} + M_E = 0 \xrightarrow{A \text{ B}} (150 \text{ kN}) 4.5 \text{ m} + M_E = 0 \xrightarrow{A \text{ B}} (150 \text{ kN}) 4.5 \text{ m} + M_E = 0 \xrightarrow{A \text{ B}} (150 \text{ kN}) 4.5 \text{ m} + M_E = 0 \xrightarrow{A \text{ B}} (150 \text{ kN}) 4.5 \text{ m} + M_E = 0 \xrightarrow{A \text{ B}} (150 \text{ kN}) \xrightarrow{A$$